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Parametric Analysis of Cross-Frame Layout Using a High-Order Beam Element

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Introduction

- Behaviour of box girder bridges
- Cross-frames
- Analysis of distortion



INTRODUCTION

The goal of the research is to provide guidelines for the design of cross-frame in box girder composite bridges against distortion

Facts

- Modern composite bridges started in the 50's and now they are frequently built
- Distortion is a well-know phenomenon which has been widely studied



Source: WSP Spain



INTRODUCTION

Motivation

 Literature usually studies simple bridges or theoretical cases



- Literature typically do not consider torsion and distortion equations coupled
- New beam finite elements
 - B3N (Cambronero 2013)
- Curvature is not usually considered in coupled torsion and distortion equations

Contribution

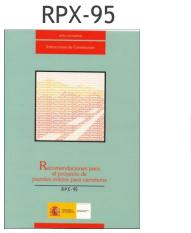
- Study of "real" bridges
 - Widths up to 24 m
 - Spans from 45 to 105 m
- Update current literature based on uncoupled equations
- Application of new finite elements to the design of cross-frames
- Include curvature in coupled torsion and distortion equations

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INTRODUCTION

Objectives

- 1. Optimize design of cross-frames
- 2. Provide useful recommendations for bridge engineers
- 3. Improve current codes
- Example: Cross-frames in codes in Spain

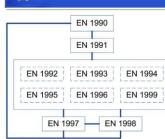


Very restrictive

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2019 DGC

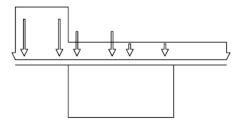




No mention



Bridges have to resist vertical loads





Source: Tschemmernegg 1989

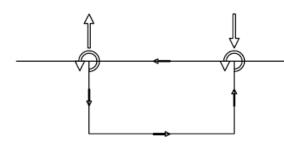
 Box girder bridges are analysed splitting up the structure in different resistant modes (Schlaich 1982)

- Longitudinal movement (Axial Forces)
- Vertical deflection (Longitudinal bending)
- Lateral deflection (Transverse Bending)
- Rotation along axis (Torsion)
- Distortion
- Transverse bending

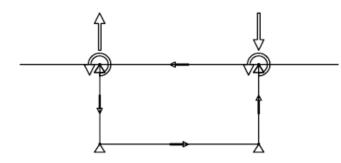
Conventional Beam Elements High-Order Beam Elements



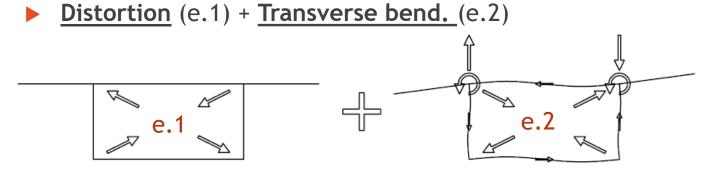
Distortion appears in box girders when external loads that causes torsion are in equilibrium with the internal torsional flux



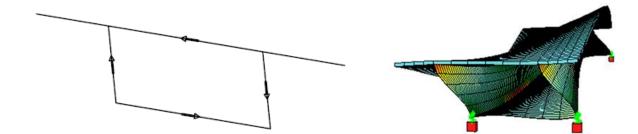
Despite the fact that it is in equilibrium it generates movements in the plane of the walls. In order to split it up the frame is analysed considering movements restricted







 e.1 Reactions in nodes generate a deflection in the plane of walls





Distortion resistance 60 _n III mechanism are: OKtop Plate action of each wall Gtop,0 (Longitudinal Stresses) Frame action r∕bot⊗ G_{bot.C} Kpot ⊗ KUMUMUMUMUMUMUMU Source: Menn 1986 **BEF** analogy p(z) Source: Pascual 2003 P(z) ΕIν ►z k₁ K_{1D}

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Composite box girder bridges

Frame action is negligible



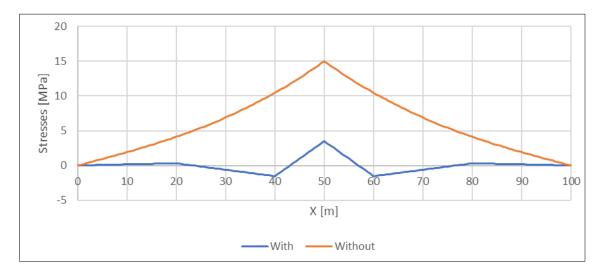
Longitudinal stresses are high



Cross-frames required in order to increase frame action and control longitudinal stresses



Stresses due to torsion and distortion in a 100 m bridge considering or not 4 full web diaphragms cross-frames





Stresses are reduced



CROSS-FRAMES

- Cross-Frames layout variables:
 - Spacing
 - Typically between 3-5 m
 - Literature recommends 4-5 cross-frames per span to control distortion
 - Туре
 - Stiffness
 - Typically design to bear all torsional loads (Very Stiff)







CROSS-FRAMES

Stresses due to torsion and distortion for different spans considering 4 full web diaphragms cross-frames per span of variable thickness

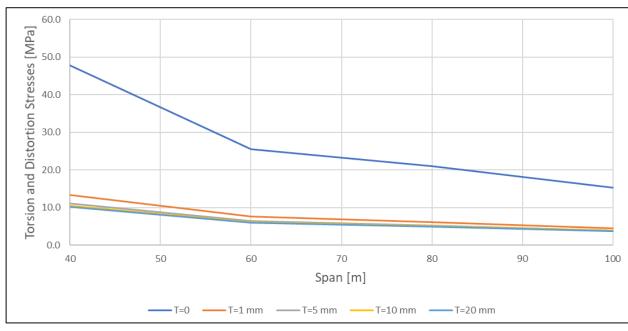
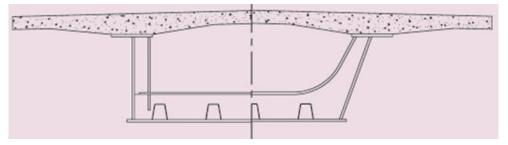


Figure 2



CROSS-FRAMES

- Objective of the research
 - Spacing
 - ► Vary between 3-5 m based in other requirements
 - Stiffness
 - Obtain the minimum to satisfy an stress limit
 - Туре
 - Study non frequent types in Spain



Source: Setra

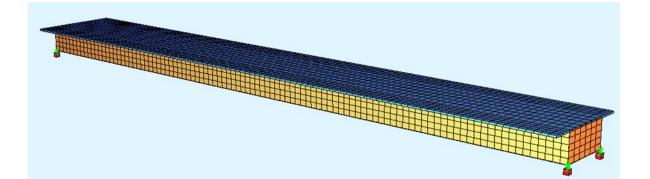


- The state of the art is based on Dabrowski and Vlasov equations:
 - Torsion (Vlasov):
 - $\blacktriangleright \quad E_0 \ I_a \ \theta_z^{IV} G_0 \ I_t \ \theta_z^{II} = m_t$
 - Distortion (Dabrowski):
 - $\blacktriangleright \quad E_0 \ K_d \ \psi_d + E_0 \ I_d \psi_d^{IV} = m_d$
- Research based on revised equations (Cambronero 2013)
 - Torsion:
 - Distortion:
 - Both equations are coupled and have been implemented in a finite element call B3N



A comparison between equations has been performed

- Reference solution: FEM model
- Vlasov and Dabrowski equations: Classic Theory
- Cambronero equations: B3N





Stresses have been obtained for different geometries

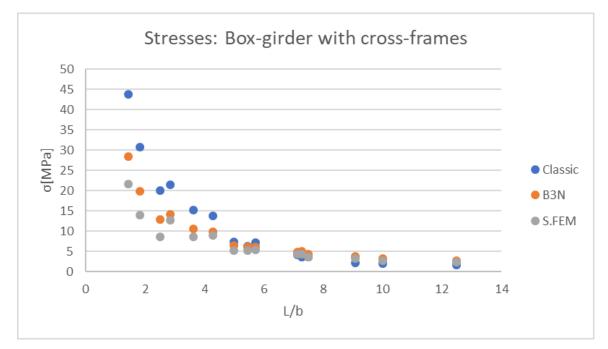
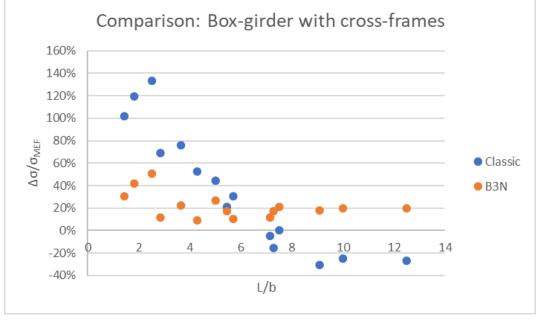


Figure 3



Stresses have been compared with FEM model

- Accuracy has been improved in B3N
- Stresses are not underestimated







THANK YOU FOR YOUR ATTENTION

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